1.- FABRICATION OF THE STEEL STRUCTURES SHALL BE PERFORMED BY SHADE STRUCTURES OR AN AUTHORIZED LICENSEE. MATERIAL TESTING (OR MILL CERTIFICATES) AND INSPECTION OF WELDING SHALL

2.- ONLY CALIFORNIA LICENSED CONTRACTORS AUTHORIZED BY SHADE STRUCTURES SHALL INSTALL THE

3.- ALL WORK SHALL CONFORM TO CBC 2016 EDITION, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).

6.- ALL PLATES PRODUCTS SHALL COMPLY WITH ASTM A572 GRADE 50.

7.- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.

8.- ALL WELDING TO CONFORM WITH AMERICAN WELDING SOCIETY STANDARDS AND SHALL BE INSPECTED BY AN AWS/CWI INSPECTOR. AWS D1.1 FOR HOT ROLLED. AWS D1.3 FOR SHEET/COLD FORMED. AWS D1.8 SEISMIC SUPPLEMENT.

9.- ALL FULL PENETRATION WELD SHALL BE CONTINUOUSLY INSPECTED PER AWS D1.1 & D1.8.

10.- SHOP CONNECTIONS SHALL BE WELDED UNLESS NOTED OTHERWISE. FIELD CONNECTIONS SHALL BE AS INDICATED ON THE DRAWINGS (IF REQUIRED). ALL FILLET WELDS SHALL BE A MINIMUM OF 3/16" ER70SX ELECTRODES UNLESS OTHERWISE NOTED. EITHER SMAW OR GMAW IS ACCEPTABLE.

11.- ALL STAINLESS STEEL BOLTS SHALL COMPLY WITH ASTM F-593, FYS = 60 KSI, FS = 95KSI ,ALLOY GROUP 1 OR 2 ALL NUTS SHALL COMPLY WITH ASTM F-594 ALLOY GROUP 1 OR 2. REFERING TO RCSC, ASTM F-593 IS NOT CONSIDERED AS HIGH STRENGTH BOLTS.

12.- ALL HIGH STRENGTH BOLTS SHALL COMPLY WITH ASTM A325 GRADE BD. ALL NUTS SHALL COMPLY WITH ASTM A-563 GRADE C. ALL HIGH STRENGTH BOLTS SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION.

13.- ALL STRUCTURAL STEEL (ITEMS FROM NOTE 5) SHALL BE PAINTED WITH ONE SHOP COAT (2.5 TO 3.5 MILS THICK MIN) OF ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT, OR EQUIVALENT PAINT SYSTEM. THIS COAT IS A WEATHER RESISTANT POWDER COATING BASED ON POLYESTER TGIC (MANUFACTURED BY SHERWIN WILLIAMS OR TIGER DRYLAC). TO ACHIEVE OPTIMUM ADHESION, IT IS RECOMMENDED THAT THE PROPER TREATMENT AND DRYING TAKE PLACE BEFORE COATING. POLYESTER POWDER (TGIC) SPECIFICATIONS SHALL BE AS FOLLOWS: - PENCIL HARDNESS (ASTM D-3363). - HUMIDITY (ASTM D-2247). - SOLVENT RESISTANCE (PCI METHOD) - 50 DBL RUBS SL. SOFTNESS.

14.- ALL STEEL ROUND TUBING (ITEMS FROM NOTE 4) SHALL BE TRIPLE COATED FOR RUST PROTECTION USING THE IN-LINE ELECTROPLATING COAT PROCESS. TUBING SHALL BE INTERNALLY COATED WITH ZINC REQUIRED. AND ORGANIC COATINGS TO PREVENT CORROSION AS MANUFACTURED BY ALLIED TUBE & CONDUIT

15.- COLD-FORMED STEEL MEMBERS SHALL BE 55% ALUMINUM ZINC ALLOY COATED PER ASTM A792/A792M STANDARD IN ACCORDANCE TO AISI \$200 TABLE A4-1, CP 90 COATING DESIGNATION. ALL EXPOSED STEEL FASTENERS, INCLUDING CAST-IN-PLACE ANCHOR BOLTS/RODS, SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM) HOT DIP GALVANIZED (ASTM A153, CLASS D MINIMUM OR ASTM F2329), OR PROTECTED WITH CORROSION PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT.

CONCRETE SPECIFICATION

1.- CONCRETE SHALL BE TESTED PER CBC 2016 SECTION 1903A & SHALL BE INSPECTED PER SECTION 1903A.

2.- CONCRETE TO BE F'c= 4500 PSI, TYPE V CEMENT, WATER/CEMENT RATIO OF 0.45, PER ACI 318-14 CHAPTER 5. REINFORCING STEEL TO BE Fy= 60000 PSI, MIN. GR. 60

3.- ALL ANCHOR BOLTS SET IN NEW CONCRETE (WHEN APPLICABLE) SHALL COMPLY WITH ASTM F-1554 GRADE 55 (GALVANIZED). ANCHOR BOLT'S EMBEDMENT NEEDS TO BE AS FOLLOW: 30 IN (MINIMUM EMBEDMENT) A) ANCHOR BOLT Ø1 1/4"

4.- CERTIFIED MILL TEST REPORTS ARE TO BE PROVIDED FOR EACH SHIPMENT OF REINFORCEMENT.

5.- ALL NON-SHRINK GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 5000 PSI, AND SHALL COMPLY THE REQUIREMENTS OF ASTM C109, ASTM C939, ASTM C1090, ASTM C1107, WHEN APPLICABLE.

FABRIC SPECIFICATION

1.- FABRIC SHALL BE MANUFACTURED BY MULTIKNIT LTD. OR OTHER COMPANY WHO CAN MANUFACTURE FABRIC, WHICH MEETS THE SPECIFICATIONS LISTED ON PAGE 2000, AND SHALL BE FABRICATED FROM POLYETHYLENE MATERIALS.

2.- THE FABRIC SHALL RETAIN 80% OF ITS TENSILE AND TEARING STRENGTH AFTER ULTRAVIOLET EXPOSURE PER ASTM G53 USING A 313 NM LIGHT SOURCE FOR 500 HOURS WHILE MOISTENED FOR 1 HOUR

3.- PROVIDE CERTIFICATION BY MANUFACTURER AND STATE FIRE MARSHALL TO DSA AT SITE SPECIFIC INSTALLATION.

4.- FABRIC SHALL REQUIRE ANNUAL INSPECTION AND MAINTENANCE BY THE DISTRICT. FABRICS SAMPLES OF THE SAME MATERIAL WHICH ARE MAINTAINED AT THE PROJECTS SITE SHALL BE TESTED TO BE IN COMPLIANCE WITH ASTM D5034 AND D2261. THE ANNUAL TESTING ON THE APPROVED PLANS SHALL BE COMPARED TO THE FABRIC SPECIFICATIONS INDICATED IN NOTE 1 OF "FABRIC SPECIFICATION" ON THE APPROVED PLANS. THE FABRIC SHALL BE REPLACED WHEN THE TEST RESULTS RETURN LESS THAN 50% OF THE ULTIMATE VALUES IN NOTE 1 OF "FABRIC SPECIFICATION".

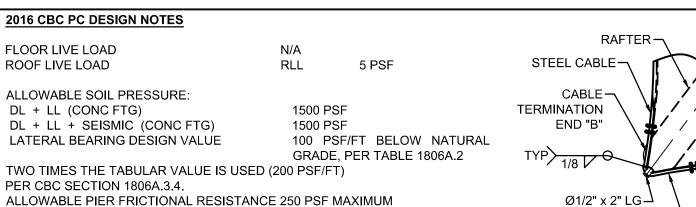
5.- FABRIC TOP NEEDS TO BE REMOVED IF SNOW EXCEEDING 5 PSF ARE ANTICIPATED, FABRIC TOP NEEDS TO BE REMOVED IF WINDS EXCEEDING 115 MPH ARE ANTICIPATED.

6.- A VISUAL INSPECTION LOOKING FOR TEAR AND ABNORMAL WEAR IN FABRIC MATERIAL AND THREAD IS REQUIRED PRIOR TO RE-INSTALLATION. SHADE STRUCTURE SHALL BE NOTIFIED IF SIGNIFICANT DAMAGE IS PRESENT BEFORE RE-INSTALLATION.

AIRCRAFT CABLE

1.- FOR FABRIC ATTACHMENT USE 3/8" 7x19 GALV. CABLE PER ASTM A1023A, ASTM 1023M-02, WITH A BREAKING STRENGTH VALUE OF 14.400 LBS. CABLE SHALL BE TENSIONED TO 250 LBS MINIMUM. THE MAXIMUM CALCULATED CABLE TENSION IS 2619 LB.

2.- CABLES SHALL BE FED THROUGH THE FABRIC SLEEVES AROUND THE PERIMETER OF THE CANOPY AND TENSIONED UNTIL THE FABRIC PANELS (DESIGNED PURPOSELY UNDERSIZED) REACH A TAUT APPEARANCE. ANY LONG TERM CABLE SAG SHALL BE MINIMIZED DURING THE MAINTENANCE RE-TIGHTING VISITS AS REQUIRED.



FLOOD HAZARD AREA WHEN A SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS

NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC

BASED ON SECTION 1810A.3.3.1.4 (ONE-SIXTH OF THE BEARING VALUE).

UPLIFT FRICTIONAL RESISTANCE HAVE A SAFETY FACTOR OF 3.

ARE STILL APPLICABLE. WIND DESIGN DIRECTIONAL PROCEDURE: ASCE 7-10, SECTION 27.4.3 -ULTIMATE DESIGN WIND SPEED (3 SEC GUST) 115 MPH -WIND EXPOSURE FACTOR -TOPOGRAPHIC FACTOR -RISK CATEGORY -VELOCITY PRESSURE EXPOSURE COEFFICIENT 0.88 -VELOCITY PRESSURE 25.32 PSF

SEISMIC DESIGN -SITE CLASS 3.00g 1.389g -SPECTRAL RESPONSE COEFFICIENTS SDS 2.00 1.39

-LATERAL FORCE RESISTING SYSTEM G.2 ORDINARY CANTILEVERED COLUMN

FIT SNUGLY INSIDE

CROSSPIECE ARM

(±1/16" TOLERANCE)

MACHINE SWAGED-

(±1/16" TOLERANCE)

BEAM-

PLATE

1" THK

DETAIL-3

1/2

EIGHT Ø1 1/4" x 36"

HVY. HEX NUTS (3)

FLAT WASHERS (2)

ANCHOR RODS

CROSSPIECE

INSIDE RIDGE

CROSSPIECE-

DETAIL-2

⊕3/16 ✓ TYP BTW. RIDGE

AND EXT. ARMS

1/2" THK

REFER TO TOP DETAIL-3

VERTICAL

-PI 1-1/4X18SC

-PROVIDE TAPER

REBAR 18#5

SPIRAL #4

TO FIT SNUGLY

-SEISMIC IMPORTANCE FACTOR -DESIGN BASE SHEAR 7194 LB -SEISMIC RESPONSE COEFFICIENTS Cs 1.6 -RESPONSE MODIFICATION FACTOR 1.25 -ANALYSIS PROCEDURE EQUIVALENT LATERAL FORCE -RISK CATEGORY -SEISMIC DESIGN CATEGORY -SITE COEFFICIENT CATEGORY Fa

GEOHAZARD REPORT IS NOT REQUIRED FOR OPEN FABRIC STRUCTURES 1,600 SQF OR LESS COMPLYING WITH THE REQUIREMENTS OF IR A-4 SECTION 3.1.1. OPEN FABRIC SHADE STRUCTURES GREATER THAN 1,600 SQUARE FEET UP TO A MAXIMUM OF 4,000 SQUARE FEET AND COMPLYING WITH THE REQUIREMENTS NOTED IN IR A-4 SECTION 3.1.1 DO NOT REQUIRE A GEOHAZARD REPORT PROVIDED A GEOTECHNICAL REPORT INDICATES THAT NO LIQUEFACTION POTENTIAL EXISTS.

ARCHITECT OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN GEOLOGIC HAZARD ZONE. GEOHAZARD REPORT REQUIREMENTS PER DSA IR A-4.

PC OPTIONS SHALL NOT INCLUDE LIQUEFIABLE SOIL (EXCEPTION: OPEN FABRIC SHADE STRUCTURES 1,600 SQUARE FEET OR LESS COMPLYING WITH REQUIREMENTS OF IR A-4 SECTION 3.1.1). IF STRUCTURE IS LOCATED IN AN FABRIC AREA WITH LIQUEFIABLE SOIL OR SITE CLASS F, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.

MINIMUM FOUNDATION SETBACK LIMIT IN ADJACENT SLOPE: THE DEPTH OF REQUIRED PIER EMBEDMENT SHALL START FROM AN ELEVATION THAT CORRESPONDS WITH A HORIZONTAL CLEAR DISTANCE OF 17'-6" THAT INTERSECT WITH THE SLOPE (DAYLIGHTING). IF SETBACK LIMITS ARE SMALLER THAN CBC REQUIRES, A SITE-SPECIFIC SOILS REPORT IS 1000

TOP VIEW

▔▕▘▔▍▀▃▝▍▀▃▀▍

2'-6"

DRILLED PIER-PIH

(EMBEDDED, PIH

(USE FOR NON-CONSTRAINED CASES)

_COLUMN

(SLRS)

PL3/8X10SQ

 VERTICAL **REBAR 18#5**

FINISHED

SURFACE

1 1/2 EXTRA

TURNS

TOP AND

BOTTOM

OF SPIRAL

MINIMUM CLASS 2 PROJECT INSPECTOR REQUIRED

Ø2'-0" SPIRAL #4

Ø13/16" HOLE -

ALL THREADED

GALVANIZED

ROD ASTM A449

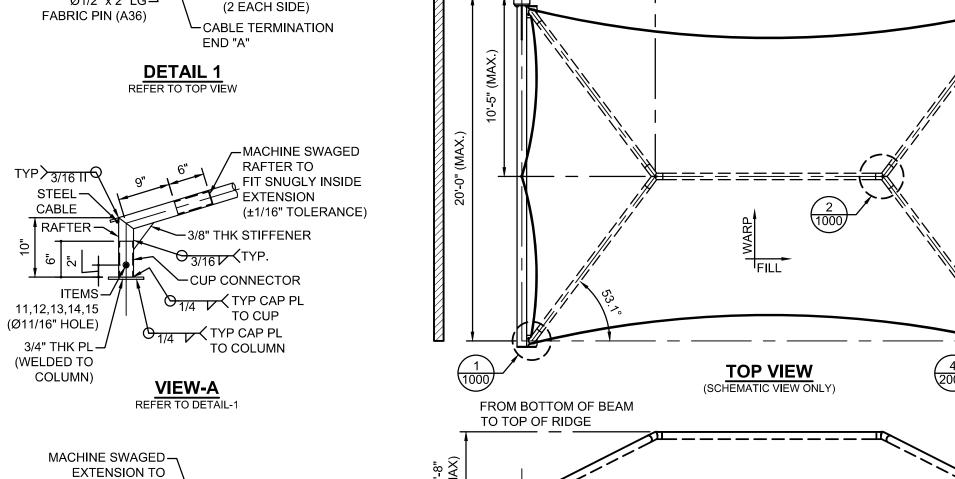
I HVY. HEX NUTS (4)

FLAT WASHERS (2)

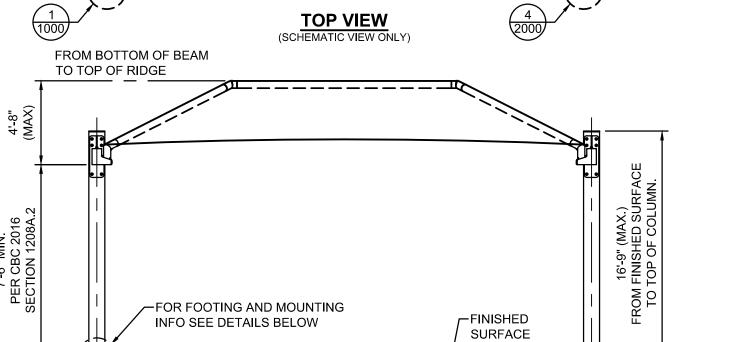
TROUGH

PROVIDE TAPER-

SLOPE 2% MAX.



7'-8" (MAX.)



FRONT VIEW

(SCHEMATIC VIEW ONLY

BUILDING

STRUCTURE

VERTICAL -

REBAR 18#5

Ø2'-0"

SPIRAL #4

-PLATE

3/4" THK

CODE ANALYSIS

TYPE

-PL3/8X10SQ

AREA

(SQ. FT.)

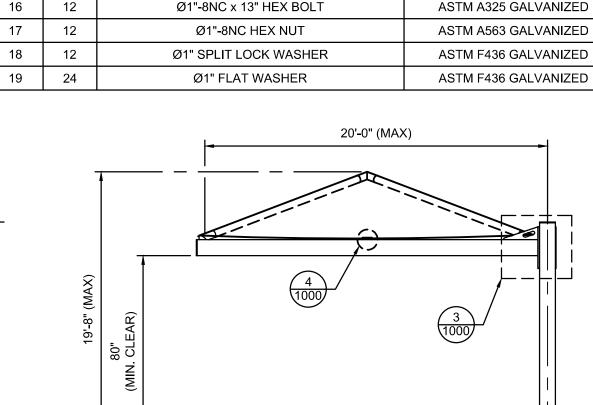
OCCUPANCY CONST.

7'-0" SQ.

STRUCTURE SHALL BE INSTALLED A MINIMUM OF 20'-0" AWAY FROM ADJACENT

BUILDING, UNLESS OTHERWISE APPROVED BY D.S.A. ON A JOB SPECIFIC BASIS

30'-0" (MAX.)



LIST OF MATERIALS

MATERIAL

HSS 10 x 10 x 0.625

HSS 10 x 6 x 0.375

HSS 10 x 6 x 0.375

HSS 4.0 x 0.25

4.50 GA 7 RD. TUBE (4.5 x 0.188)

FR COLOURSHADE Z25

GALVANIZED STEEL

GALVANIZED STEEL

18**-**8 SS

18**-**8 SS

18**-**8 SS

DELRIN (ACETAL)

DESCRIPTION

COLUMN

BEAM LEFT

BEAM RIGHT

CUP CONNECTOR (6" LG)

RAFTER (GALVANIZED STEEL TUBE)

EXTENSION (GALVANIZED STEEL TUBE)

CROSSPIECE (GALVANIZED STEEL TUBE)

RIDGE

FABRIC TOP

Ø3/8" CABLE

Ø3/8" CABLE CLAMP

Ø5/8"-11NC x 6" HEX BOLT

Ø5/8"-11NC HEX NUT

Ø5/8" FLAT WASHER

Ø5/8" FLAT WASHER

Ø5/8" SPLIT LOCK WASHER

ITEM QTY

2A | 1

2B 1

6 | 2

8 | 1

10 4

11 | 4

12 4

13 | 8

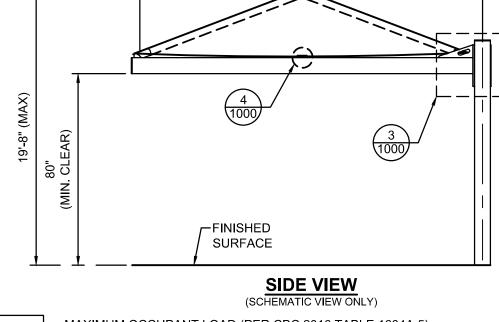
14

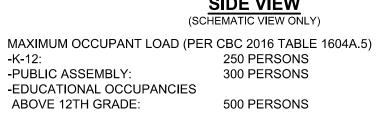
15

OCCUPANT OCCUPANT

LOAD

LOAD FACTOR





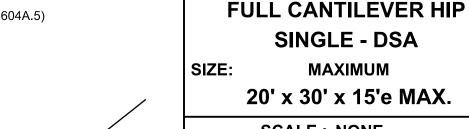
7'-0" SQ.

1'-6" SQ.

--PL1-1/4X18SQ

Ø2'-0"

SPIRAL #4



SCALE: NONE

THESE PLANS AND SPECIFICATIONS ARE THE

PROPERTY OF USA SHADE AND FABRIC

STRUCTURES AND SHALL NOT BE

REPRODUCED WITHOUT THEIR WRITTEN

& Fabric Structures

PERMISSION.

CORPORATE HEADQUARTERS

8505-A CHANCELLOR ROW

DALLAS, TX, 75247

800-966-5005

IAS CERTIFICATION No: FA-428

CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

Solana Beach Elementary

CERTIFICATIONS:

School District

Solana Pacific

Elementary School

3901 Townsgate Dr.

San Diego, CA 92130

DSA2022030-16

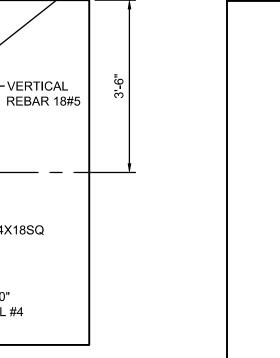
PROJECT NAME:

MODEL NUMBER:

STRUCTURE TYPE:

CUSTOMER:

DRAWING SIZE:



IDENTIFICATION STAMP DIVISION OF THE STATE ARCHITEC APP. NO: 04 - 117140 INCR: AC <u>DF</u> FLS <u>DS</u> SS <u>VN</u> DATE <u>08/14/2018</u>

08/10/2018 10:08:26 AM

PRE-CHECK (PC) DOCUMENT Code : 2016 CBC A separate project application for construction is required.

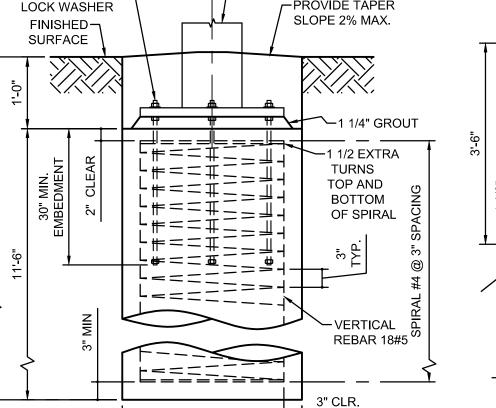
02/26/18 Eng. By : 02/26/18 Design By: Approved By: JO 02/26/18 **DRAWING DESCRIPTION:**

PRODUCT INFORMATION

DSA2022030-16

9.1-1000 REV.

TOP VIEW TOP VIEW PROVIDE TAPER -COLUMN (SLRS) STD. SLOPE 2% MAX SECOND POUR EIGHT Ø1 174," x 36"-Ø3/4"X20" -ANCHOR RODS -COLUMN ALL THREADED HVY. HEX NUTS (3) -PROVIDE TAPER -FINISHED (SLRS) ROD ASTM A449 FLAT WASHERS (2) SURFACE STD. SLOPE 2% MAX GALVANIZED LOCK WASHER HVY. HEX NUTS (4) -FINISHED FLAT WASHERS (2) SURFACE •/• • • • • | //>// 1/4" GROUT TURNS ニ コ = ー lo b 片 | TOP AND **↓** TURNS воттом #=#=# Top AND OF SPIRAL , волтом REBAR E.W. =#¶─┼OFSPARAL -REBAR E.W. Ş TOP AND TOP AND BOTTOM BOTTOM 12#5 ==<u>=</u>=<u>-</u>-<u>-</u> 3" CLEAR ►CAP PLATE 3" CLEAR ALTERNATE SPREAD FOOTING ALTERNATE SPREAD FOOTING



2'-6"

DRILLED PIER-RBP

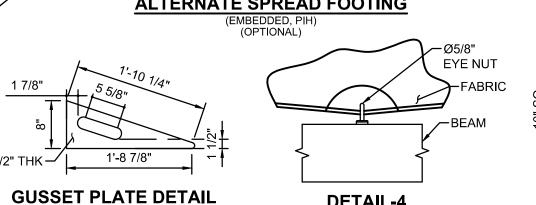
RECESSED BASE PLATE. RBP (USE FOR NON-CONSTRAINED CASES)

(OPTIONAL)

TOP VIEW

_COLUMN

(SLRS)



DETAIL-4 (TYP. FOR ALL CÓLUMNS (TOP OF RBP COLUMNS

REFER TO SIDE VIEV

CAP PLATE

(TOP & BOT. OF PIH COLUMNS)

(RECESSED BASE PLATE, RBP)

(OPTIONAL)

ENVELOPE JOINT REACTIONS

Shear resultant = $\sqrt{Px^2 + Py^2 + Pz^2}$

Moment resultant = $\sqrt{Mx^2 + My^2 + Mz^2}$

							ASD REACTION	ONS				
Node	Support Forces [kip]			Support Moments [kipft]				Support Forces [kip]	Support Moments [kipft]	Support Forces [kip]	Support Forces [kip	
No.		P _{X'}	P _{Y'}	P _{Z'}	M _{X'}	M _{Y'}	M _Z		SHEAR RESULTANT	MOMENT RESULTANT	UPLIFT	AXIAL
						MAXI	I MUM REAC	TIONS	3.638	54.772	0.078	-4.455
lode			Support Forces [kip		Sup	pport Moments [ki						9
No.		P _X	P _Y	P _z	M _X	M _Y	M _z					
86	Max	3.611	3.317	0.078	19.232	42.920	3.519					
	Min	-0.391	-1.053	-4.455	-54.637	-7.079	-13.558					
	Max P _{x'}	3.611	0.445	-0.631	-11.745	42.920	-12.489	CO 24	3.638	44.498		-0.631
	Min P _{x'}	-0.391	-1.053	-2.901	-11.832	-7.079	-9.837	CO 39	1.123	13.788		-2.901
	Max P _{Y'}	0.382	3.317	-0.729	-43.866	5.527	-2.058	CO 25	3.339	44.213		-0.729
	Min P _{Y'}	-0.391	-1.053	-2.901	-11.832	-7.079	-9.837	CO 39	1.123	13.788		-2.901
	Max P _{z'}	1.260	0.788	0.078	-0.643	16.523	-11.156	CO 41	1.486	16.536	0.078	
	Min P _{Z'}	0.714	0.637	-4.455	-40.099	13.111	2.387	CO 15	0.957	42.188		-4.455
3	Max M _{x'}	1.729	-0.901	0.063	19.232	24.868	-6.617	CO 42	1.950	31.437	0.063	
3	Min M _{X'}	0.250	3.317	-2.916	-54.637	3.840	-1.374	CO 23	3.326	54.772		-2.916
	Max M _{Y'}	3.611	0.445	-0.631	-11.745	42.920	-12.489	CO 24	3.638	44.498		-0.631
	Min M _Y	-0.391	-1.053	-2.901	-11.832	-7.079	-9.837	CO 39	1.123	13.788		-2.901
	Max M _{z¹}	1.069	0.914	-3.019	-32.402	18.473	3.519	CO 38	1.406	37.298		-3.019
	Min M _{z'}	1.350	0.635	0.003	1.860	17.424	-13.558	CO 6	1.492	17.523	0.003	
88	Max	3.156	3.317	0.078	19.216	36.750	13.559					
	Min	-1.950	-1.053	-4.455	-54.637	-28.102	-7.424					
	Max P _{x'}	3.156	-0.446	-3.011	-6.154	36.750	-7.424	CO 22	3.187	37.262		-3.011
	Min P _{X'}	-1.950	-0.707	0.008	14.556	-28.102	7.515	CO 7	2.074	31.648	0.008	
	Max P _{Y'}	-0.382	3.317	-0.729	-43.866	-5.527	2.058	CO 25	3.339	44.213		-0.729
	Min P _{y'}	0.392	-1.053	-2.901	-11.834	7.082	9.835	CO 39	1.124	13.791		-2.901
	Max P _{z'}	-1.259	0.787	0.078	-0.616	-16.519	11.156	CO 41	1.485	16.530	0.078	
	Min P _{z'}	-0.714	0.638	-4.455	-40.098	-13.109	-2.388	CO 15	0.958	42.186		-4.455
	Max M _{x'}	-1.729	-0.900	0.064	19.216	-24.863	6.613	CO 42	1.949	31.423	0.064	
	Min M _{x'}	-0.250	3.317	-2.916	-54.637	-3.840	1.374	CO 23	3.326	54.772		-2.916
	Max M _{y'}	3.156	-0.446	-3.011	-6.154	36.750	-7.424	CO 22	3.187	37.262		-3.011
	Min M _{y'}	-1.950	-0.707	0.008	14.556	-28.102	7.515	CO 7	2.074	31.648	0.008	
	Max M _{z'}	-1.350	0.635	0.003	1.876	-17.422	13.559	CO 6	1.492	17.523	0.003	
	Min M _{Z'}	3.156	-0.446	-3.011	-6.154	36.750	-7.424	CO 22	3.187	37.262	2.000	-3.011

BASIC LOAD CASES

DEAD LOAD 0.0378 PSF (FABRIC)

FLOOR LIVE LOAD ROOF LIVE LOAD ROOF SNOW LOAD

5 PSF 5 PSF N/A SUPERIMPOSED LOADS

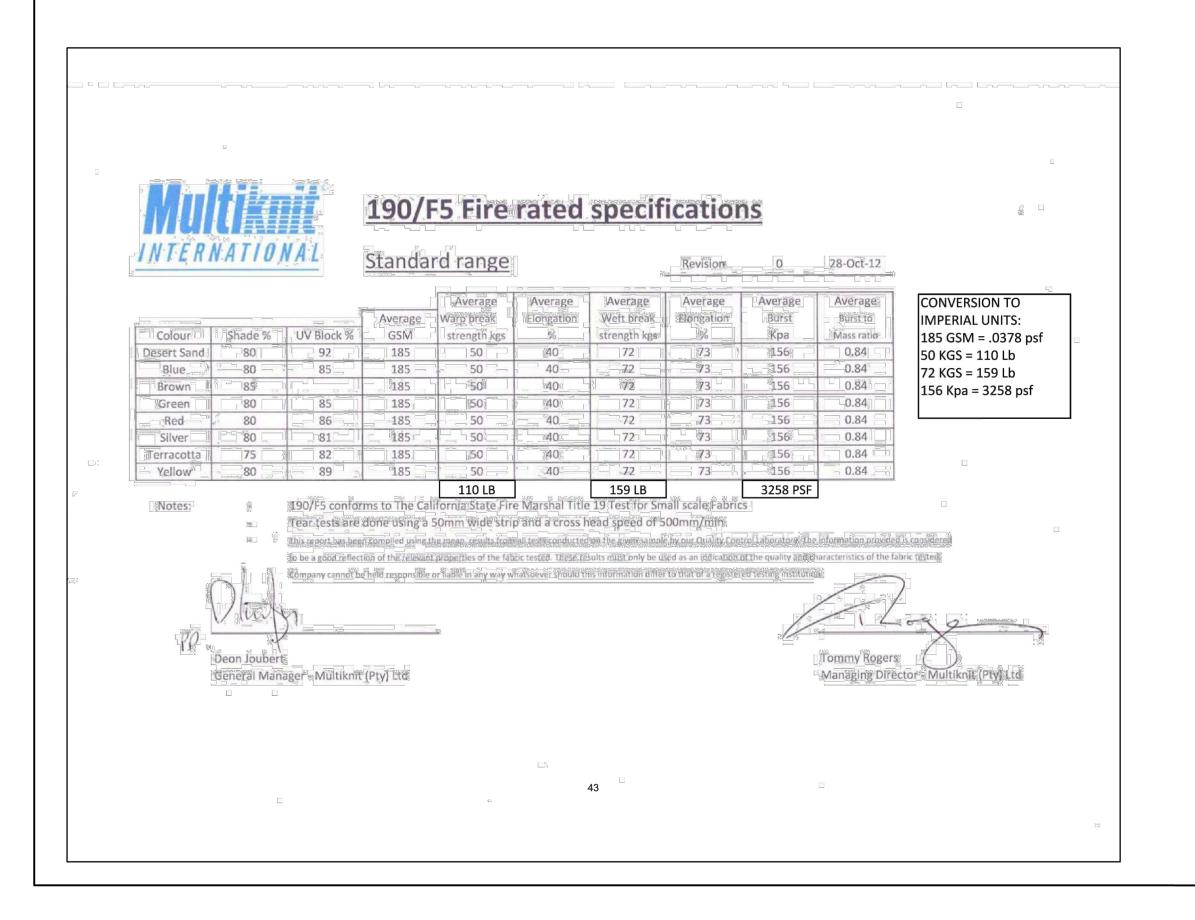
N/A

WIND LOAD

ULTIMATE DESIGN WIND SPEED (3 SEC GUST) 115 MPH VELOCITY PRESSURE qz

COMPONENT AND CLADDING qz (CABLE AND CABLE HARDWARE ONLY) 25.32 PSF

SEISMIC RESPONSE COEFFICIENTS Cs 1.6 DESIGN BASE SHEAR







A forged fabrication allows these to be used in critical applications such as tie downs and support lines. They must be oriented with the saddle on the long (live) end and U-bolt on the short (dead) end. Also known as wire rope clips.

Galvanized steel clamps have a thick coating for corrosion resistance.

316 stainless steel clamps are the most corrosion resistant fittings we offer. They provide excellent resistance to salt water and Warning: Test all assemblies for required strength before use. Do not use with coated rope unless the coating is removed.

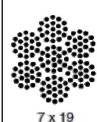
Dead End-Live End

					- Clamp -			
For Rope	No. of Clamps	Rope	Required					
Dia.	Required	Turnback	Torque, ftlbs.	Ht.	Wd.	Thick.	Capacity	Specifications Met
Galvanized	I Steel							
1/8"	2	3 1/4"	4.5	1 1/8"	1"	13/16"	80% of the Rope's Capacity	
3/16"	2	3 3/4"	7.5	1 1/2"	1 3/16"	1"	80% of the Rope's Capacity	_
1/4"	2	4 3/4"	15	1 314"	1 7/16"	1 1/4"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
5/16"	2	5 1/4"	30	2 1/8"	1 11/18"	1 5/16"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
3/8"	2	6 1/2"	45	2 7/16"	2"	1 11/18"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
7/16	2	1"	65	3 1/16"	2 5/16"	1 15/18"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
1/2"	3	11 1/2"	65	3 1/16"	2 5/16"	1 15/16"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
9/16"	3	12"	95	3 58"	2 1/2"	2 1/16"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
5/8"	3	12"	95	3 5%"	2 1/2"	2 1/16"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
3/4"	4	18"	130	4 3/16"	2 7/8"	2 144"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
7/8"	4	19"	225	4 3/4"	3 3/16"	2 1/2"	80% of the Rope's Capacity	Fed. Spec. FF-C-450
1"	5	26"	225	5 5/16"	3 1/2"	2 11/18"	90% of the Rope's Capacity	Fed. Spec. FF-C-450
1 1/8"	6	34"	225	5 13/10"	3 5/8"	2 13/16"	90% of the Rope's Capacity	Fed. Spec. FF-C-450
1 1/4"	7	44"	36D	6 5/8"	4 3/16"	3 3/16"	90% of the Rope's Capacity	Fed. Spec. FF-C-450
1 3/8"	7	44"	360	6 3/4"	4 1/4"	3 3/16"	90% of the Rope's Capacity	Fed. Spec. FF-C-450
1 1/2"	8	54"	360	7 7/16"	4 1/2"	3 7/16"	90% of the Rope's Capacity	Fed. Spec. FF-C-450

Aircraft Cable

Preformed, made in accordance with commercial specifications military and federal specification rope available.

Carbon Steel (Aircraft Cable) - Galvanized cable has the highest strength and greatest fatigue life of the materials offered. It has good to fair corrosion resistance in rural to industrial atmosphere environments. This material is most widely used for small diameter cables. Tin over galvanized cable offers greater corrosion resistance and reduced friction over pulleys.



	7 x 1	Galvanized Min.		
	Dia. (In)	Approx. Wt 1000 Ft/lbs	Breaking Strengths (lbs)	
7 x 19	3/32	17.	1,000	
	1/8	29.	2,000	
	5/32	45.	2,800	
	3/16	65.	4,200	
	7/32	86.	5,600	
	1/4	110.	7,000	
	9/32	139.	8,000	
	5/16	173.	9,800	
	3/8	243.	14,400	

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CORPORATE HEADQUARTERS 8505-A CHANCELLOR ROW DALLAS, TX, 75247 800-966-5005

CERTIFICATIONS:

IAS CERTIFICATION No: FA-428 CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:

Solana Beach Elementary

School District

PROJECT NAME:

Solana Pacific

Elementary School

LOCATION: 3901 Townsgate Dr.

San Diego, CA 92130

MODEL NUMBER:

DSA2022030-16

STRUCTURE TYPE:

FULL CANTILEVER HIP SINGLE - DSA

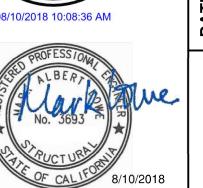
MAXIMUM

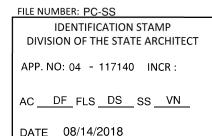
20' x 30' x 15'e MAX.

SCALE: NONE

DRAWING SIZE:









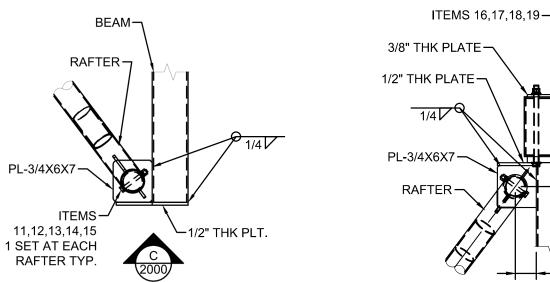
Eng. By :	JO	02/26/18			
Design By :	MP	02/26/18			
Approved By :	JO	02/26/18			
DDAWING DECODIDION					

DRAWING DESCRIPTION:

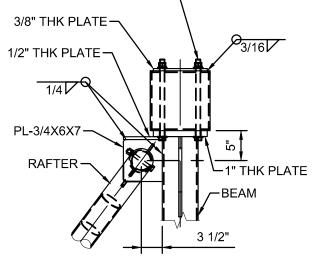
REACTIONS

DSA2022030-16

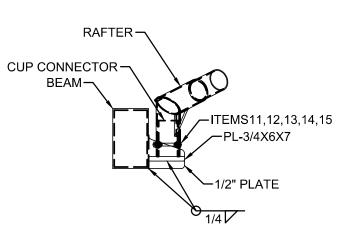
9.2-2000







DETAIL-5



VIEW C